Planning Proposal Proposed Mixed Use Development

124-142 Beamish Street, Campsie

TRAFFIC AND PARKING ASSESSMENT REPORT

3 April 2020

Ref 18718



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1. INTRODUCTION

This report has been prepared to accompany a planning proposal for a mixed-use development to be located at 124-142 Beamish Street, Campsie (Figures 1 and 2)

The planning proposal seeks approval to amend the planning controls in order to permit a mixed-use development with an increased apartment yield. Off-street car parking will ultimately be provided in a new multi-level basement car parking area beneath the building in accordance with *SEPP 65* and Council requirements.

The site is located within the heart of the "Campsie Station Precinct" as outlined in the Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document. The document details the redevelopment objectives of the corridor and comprises a range of uses, including low/medium/high residential, retail/commercial premises, schools and recreational facilities. The document also identifies improvements to alternate forms of travel including pedestrian and cycle paths as well as public transport.

The purpose of this report is to assess the traffic and parking implications of the planning proposal and to that end this report:

- describes the site and provides details of the planning proposal
- reviews the road network in the vicinity of the site, and the traffic conditions on that road network
- reviews the alternate forms of transport available in the vicinity of the site
- estimates the traffic generation potential of the planning proposal, and assigns that traffic generation to the road network serving the site
- assesses the traffic implications of the development proposal in terms of road network capacity

- reviews the geometric design features of the proposed car parking and loading facilities for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking and loading envisaged on the site.





2. PLANNING PROPOSAL

Site

The subject site is located on the western side of Beamish Street, extending between Ninth Avenue and Campsie Street. The site occupies an area of approximately 3,845m² and has street frontages of approximately 75m in length to Ninth Avenue, 52m in length to Beamish Street and 60m in length to Campsie Street.

The subject site is currently occupied by a mix of two-storey retail and commercial buildings comprising a cumulative floor area of approximately 4,745m².

At-grade off-street parking is provided at the rear of the buildings, with vehicular access provided via driveways located off the Campsie Street, Beamish Street and Ninth Avenue site frontages.

A recent aerial image of the site and its surroundings is reproduced below.



Existing Planning Controls

The primary instrument that governs the mass and scale of the development on the site are contained within the *Canterbury Local Environment Plan 2012 (CLEP2012)*.

The subject site is zoned B2 - Local Centre and is not subject to any FSR control. The scale of any development on the site is currently constrained by a building height of 21m.

It is therefore envisaged that a mixed-use development comprising 240 apartments, $3,000m^2$ of retail floor space and $2,500m^2$ of commercial floor space is achievable under the existing planning controls of the site.

Sydenham to Bankstown Urban Renewal Corridor

As outlined in the Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document, the NSW Government plans for 35,400 new homes and 8,700 new jobs over the next 20 years and infrastructure to support the future community's needs. The strategy builds on the Sydney Metro City and Southwest project and provides a co-ordinated approach to infrastructure delivery and development across the corridor, with the project's priorities summarised below:

- provide a range of new homes to suit different lifestyles and budgets
- allow for new homes to be built close to the metro stations
- retail streets that contribute to the local character of the area and protect heritage areas
- retail the scale and character of popular local shopping areas
- ensure adequate schools and childcare centres are provided for the future increased population
- identify areas for pedestrian and cycle paths including the potential for GreenWay South West
- provide a significant increase in transport capacity with new, faster and more frequent services

Campsie Station Precinct

The site is located within the heart of the "Campsie Station Precinct" as outlined in Department of Planning & Environment's *Sydenham to Bankstown Urban Renewal Corridor Strategy* document.

The document details the redevelopment objectives of the corridor and comprises a range of uses, including low/medium/high residential, retail/commercial premises, schools and recreational facilities. The document also identifies improvements to alternate forms of travel including pedestrian and cycle paths as well as public transport.





22 Sydenham to Bankstown Urban Renewal Corridor Strategy

The Sydney Metro works include improvement to the existing entry of the railway station along with public domain and access improvements, with increased footpath widths which will encourage local retail activity surrounding the railway station.

Along with the new Metro service, the strategy proposes to introduce bus priority measures along Beamish Street, Seventh Avenue and Fifth Avenue for the 409 and M41 services, while increased service levels are also proposed for the 487 service between Canterbury local centre and Bankstown CBD. The modal changes between rail and bus networks will also be improved with an upgraded interchange at Campsie railway station.

The completion of the western "Campsie Bypass" at Loch Street will improve regional vehicle and bus connections to major roads and employment destinations and pedestrian amenity along Beamish Street.

Alternate forms of transport including walking, cycling and public transport will be further encouraged by improving the quality of the railway station for passengers with better access, facilities and signage as well as improving pedestrian and cycle access within the precinct. A new east-west regional cycle link along the rail corridor is also planned in order to improve pedestrian and cycle access between the town centres and railway stations along the corridor.

New streets and/or pedestrian connections are also proposed through larger blocks as they develop, enhancing the permeability of the current vehicular and pedestrian network within the Campsie Station Precinct, including a new link between Ninth Avenue and Campsie Street.

Planning Proposal

The planning proposal seeks approval to amend the current planning controls which apply to the site in order to increase the site's development yield, permitting a mixed-use development comprising buildings ranging in height from 4 storeys to 25 storeys.

For the purposes of this traffic assessment it has been assumed that the proposed mixed-use building could comprise in the order of 320 apartments on the upper levels as follows:

TOTAL APARTMENTS:	320
3 bedroom apartments:	25
2 bedroom apartments:	161
1 bedroom apartments:	134

In addition, approximately $3,145m^2$ of retail floor space and approximately $1,600m^2$ of commercial floor space is also proposed on the lower levels.

Off-street car parking will ultimately be provided in a new multi-level basement car parking area beneath the buildings, designed to comply with *SEPP 65* and Council's requirements as well as the relevant Australian Standards. Vehicular access to the site is proposed to be provided via a new entry/exit driveway to be located at the western end of the Ninth Avenue site frontage.

Loading/servicing for the proposed development is expected to be undertaken by a variety of commercial vehicles up to and including 8.8m long medium rigid trucks. A dedicated loading area is to be located on the ground level at the rear of the retail tenancies and fitted with a mechanical turntable, thereby allowing all trucks to enter and exit the site in a forward direction at all times. Vehicular access to the loading area is to be provided via a dedicated service driveway which is to be located adjacent to the abovementioned proposed basement site access driveway off Ninth Avenue.

Plans for the purposes of this planning proposal have been prepared by *Turner Architects* and are reproduced in the following pages.















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3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services is illustrated on Figure 3.

Canterbury Road is classified by the RMS as a *State Road* and provides the key east-west road link in the area, linking Bankstown and Hurlstone Park. It typically carries two traffic lanes in each direction in the vicinity of the site, with clearway restrictions applying along both sides of the road during commuter peak periods.

Beamish Street and Brighton Avenue is classified by the RMS as a *Regional Road* which provides the key north-south road link in the area, linking Canterbury Road to Georges River Road. The route typically carries one traffic lane in each direction in the vicinity of the site and kerbside parking is permitted at selected locations along the road.

Ninth Avenue is also classified by the RMS as a *Regional Road* which provides the key eastwest road link in the area, linking Campsie to Wiley Park via Albert Street and Lakemba Street. It typically carries one traffic lane in each direction in the vicinity of the site and kerbside parking is generally permitted on one or both sides of the road, subject to sign posted restrictions.

Campsie Street is a local, unclassified road which is primarily used to provide vehicular and pedestrian access to frontage properties. Kerbside parking is generally permitted along both sides the road, subject to sign posted restrictions.

Existing Traffic Controls

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

 a 40 km/h SPEED LIMIT which applies to Beamish Street and all other local roads in the Campsie City Centre





- TRAFFIC SIGNALS in Beamish Street where it intersects with Ninth Avenue, Clissold Parade and also South Parade/Lilian Street
- RIGHT TURN HOLDING BAYS in Beamish Street where it intersects with Ninth Avenue and also Campsie Street
- a NO RIGHT-TURN westbound restriction in Ninth Avenue onto Sixth Avenue
- a LEFT-TURN ONLY southbound restriction in Sixth Avenue onto Ninth Avenue
- RAISED PEDESTRIAN CROSSINGS in Campsie Street and also North Parade in the vicinity of Beamish Street.

Existing Public Transport Services

The existing public transport services available to the site are illustrated on Figure 5.

The subject site is conveniently located within 200 metres of walking distance to the entrance of Campsie Railway Station. The station lies on the T3 Bankstown Line which operates between Bankstown and Sydney CBD. Services generally operate at a frequency of approximately 5-10 minutes during commuter peak periods and 15-30 minutes at other times.

In addition to the train services, a number of bus routes currently operate along Beamish Street with bus stops located within a 100 metres walking distance from the subject site. Notably, the M41 bus route currently operates along Beamish Street which is part of the Sydney's *Metrobus* network that provides high-frequency, high-capacity links between key employment and growth centres across Sydney. The M41 links between Hurstville, Bexley North, Campsie, Burwood, Concord, Rhodes, Ryde, Top Ryde, North Ryde, Macquarie park and Macquarie Centre operating at 10-minute intervals during commuter peak periods, 15-minute intervals during the day and 20-minute intervals at other times.

Other bus services available in the vicinity of the subject site includes route 400, 412, 413, 415, 444, 445, 473, 487, 490, 492 and 942.



The site lies within the heart of the Campsie town centre which includes a wide range of essential shops and services such as a mini-market, fruit market, butchery, bakery, seafood shop, bottle shop, post office, pharmacy, optometrist, newsagency, hair dresser and beautician.

The site is therefore considered to be highly accessible to essential services and public transport options.

Local Bicycle Routes

The location of the existing and proposed bicycle routes in the vicinity of the site are illustrated on the figure below, with the subject site marked with a star. These bicycle routes are readily accessible from the subject site and provide a number of on-road and off-road bicycle routes linking the local area with the greater surrounding area. The proposed development will make provision for a substantial bicycle parking area which is to be located within the basement level and will enhance the *active* transport options available to future occupants of the site.



Sydney Metro City & Southwest

As described on the Transport for NSW website, the new Sydney Metro City & Southwest rail line is one of the NSW Government's largest infrastructure projects being delivered to serve a growing Sydney. Regular services will be provided from early morning to late evening, including every 4 minutes during peak periods, and there will not be a timetable, customers simply turn up and go.

Stage 1 "Norwest" will deliver a new 36km line, connecting Cudgegong Road to Chatswood and include 8 new metro stations, five upgraded stations and 4,000 commuter car parking spaces. Stage 1 is expected to open in the first half of 2019.

Stage 2 "City & Southwest" will deliver a 30km extension of the metro rail from Chatswood under Sydney Harbour, through new CBD stations and south-west to Bankstown. Stage 2 is due to open in 2024, with 7 new metro stations and 11 upgraded stations.



Sydney Metro alignment map

Existing Traffic Conditions

An indication of the existing traffic conditions on the road network in the vicinity of the site is provided by peak period traffic surveys undertaken as part of this traffic study. The traffic surveys were during the morning and afternoon peak periods on Thursday 1st November, 2018 at the following intersections:

- Beamish Street and Ninth Avenue intersection
- Beamish Street and Campsie Street intersection.

The results of the traffic surveys are reproduced in full in Appendix A and reveal that:

- two-way traffic flows in Beamish Street are typically in the order of 1,300 vehicles per hour (vph) during peak periods
- two-way traffic flows in Ninth Avenue are typically in the order of 1,000 vph during peak periods
- two-way traffic flows in Campsie Street are typically in the order of 250 vph during peak periods.

Projected Traffic Generation

An indication of the traffic generation potential of the planning proposal is provided by reference to the Roads and Maritime Services publication *Guide to Traffic Generating Developments, Section 3 - Landuse Traffic Generation (October 2002)* and the updated traffic generation rates in the recently published RMS *Technical Direction (TDT 2013/04a)* document.

The *TDT 2013/04a* document specifies that it replaces those sections of the RMS *Guidelines* indicated, and that it must be followed when RMS is undertaken trip generation and/or parking demand assessments.

The RMS *Guidelines* and the updated *TDT 2013/04a* are based on extensive surveys of a wide range of land uses and nominate the following traffic generation rates which are applicable to the planning proposal:

Commercial Offices

- AM: 1.6 peak hour vehicle trips per 100m² GFA
- PM: 1.2 peak hour vehicle trips per $100m^2$ GFA

High Density Residential Flat Dwellings

- AM: 0.19 peak hour vehicle trips per unit
- PM: 0.15 peak hour vehicle trips per unit

The RMS *Guidelines* also make the following observation in respect of high density residential flat buildings:

Definition

A *high density residential flat building* refers to a building containing 20 or more dwellings. This does not include aged or disabled persons housing. *High density residential flat buildings* are usually more than 5 levels, have basement level car parking and are located in close proximity to public transport services. The building may contain a component of commercial use.

Factors

The above rates include visitors, staff, service/delivery and on-street movements such as taxis and pick-up/set-down activities.

However, the RMS *Guidelines* and the updated *TDT 2013/04a* do not nominate a traffic generation rate for small, local shops, referring only to major regional shopping centres incorporating supermarkets and department stores. For the purpose of this assessment therefore, the commercial traffic generation rate has been adopted in respect of the retail component of the planning proposal.

Application of the above traffic generation rates to the various components of the development proposal yields a traffic generation potential of approximately 137 vph during the weekday AM peak period and approximately 105 vph during the weekday PM peak period, as set out below:

	AM	PM
Residential (320 apartments):	61 vph	48 vph
Retail shops (3,145m ²):	50 vph	38 vph
Commercial offices (1,600m ²):	26 vph	19 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	137 vph	105 vph

Planning Proposal - Projected Future Traffic Generation Potential

That projected future level of traffic generation potential which is expected to occur as a consequence of the planning proposal should however, be offset or *discounted* by the volume of traffic which could reasonably be expected to be generated by a development permitted by the current *CLEP2012* planning controls (in terms of height).

Application of the above traffic generation rates to the 240 residential apartments, 3,000m² of retail floor space and 2,500m² of commercial floor space achievable under the current *LEP 2012* planning controls yields a traffic generation potential of approximately 134 vph during the weekday AM peak period and approximately 102 vph during the weekday PM peak period, as set out below:

Current CLEP 2012 Planning Controls - Projected Future Traffic Generation Potential

	AM	PM
Residential (240 apartments):	46 vph	36 vph
Retail shops (3,000m ²):	48 vph	36 vph
Commercial offices (2,500m ²):	40 vph	30 vph
TOTAL TRAFFIC GENERATION POTENTIAL:	134 vph	102 vph

Accordingly, it is likely that the planning proposal will result in a *nett* increase in the traffic generation potential of the site of just 3 vph during the weekday AM and PM peak periods, when compared with the existing planning controls that apply to the site, as set out below:

Projected Nett Increase in Peak Hour Traffic Generation Potential
of the Site as a Consequence of the Planning Proposal

	AM	PM
Planning Proposal Traffic Generation Potential:	137 vph	105 vph
Existing Planning Controls Traffic Generation Potential:	-134 vph	-102 vph
NETT INCREASE IN TRAFFIC GENERATION POTENTIAL:	+3 vph	+3 vph

Notwithstanding, for the purposes of this assessment it has been assumed that *all* of the projected future traffic flows of 137 vph and 105 vph in the AM and PM commuter peak periods respectively, will be new or *additional* to the existing traffic flows currently using the adjacent road network.

That projected increase in the traffic generation potential of the site as a consequence of the planning proposal will not have any unacceptable traffic implications in terms of road network capacity, as is demonstrated by the following section of this report.

Traffic Implications - Road Network Capacity

The traffic implications of planning proposals primarily concern the effects that any *additional* traffic flows may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA NETWORK 8 program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages. The Movement Summaries for the traffic analysis are reproduced in Appendix B.

The results of the SIDRA NETWORK analysis of the intersections are summarised in Tables 3.1 and Table 3.2 on the following pages, revealing that:

Beamish Street and Ninth Avenue Intersection (Table 3.1)

- the intersection currently operates at *Levels of Service "A" & "B"* under the existing traffic demands during the commuter peak periods with total average vehicle delays in the order of 14-15 seconds/vehicle
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection is expected to continue to operate at *Level of Service "B"* during the commuter peak periods, with increases in total average vehicle delays of *less than* 1 second/vehicle.

TABLE 3.1 - RESULTS OF SIDRA ANALYSIS OF BEAMISH STREET & NINTH AVENUE					
Var Indiastans		Existing Traffic Demand		Projected Development Traffic Demand	
		AM	PM	AM	РМ
Level of Service		В	А	В	A
Degree of Saturation		0.787	0.797	0.839	0.827
Average Vehicle Delay (secs/veh)					
Beamish Street (south)	L T	7.7 17.9	7.6 15.4	77 17.9	7.7 15.4
Beamish Street (north)	T R	7.6 16.3	6.5 14.6	7.6 16.6	6.5 14.9
Ninth Avenue (west)	L R	12.1 26.9	12.8 28.6	12.2 29.5	12.8 30.0
TOTAL AVERAGE VEHICLE	DELAY	14.8	14.0	15.4	14.3
		BEA	NINX	BEA	_NINP

Beamish Street and Campsie Street Intersection (Table 3.2)

- the intersection currently operates at Level of Service "A" under the existing traffic demands during the commuter peak periods with total average vehicle delays in the order of 1 second/vehicle. Notwithstanding, the right turn movement out of Campsie Street onto Beamish Street experience delays in the order of 20 seconds/vehicle.
- under the projected future traffic demands expected to be generated by the planning proposal, the intersection is expected to continue to operate at Level of Service "A" during the commuter peak periods, with no change in total average vehicle delays, whilst the right turn movement out of Campsie Street onto Beamish Street increases by just 1 second/vehicle.

TABLE 3.2 - RESULTS OF SIDRA ANALYSIS OFBEAMISH STREET & CAMPSIE STREET					
		Existing Traffic Demand		Projected Development Traffic Demand	
Key mulcators		AM	PM	AM	PM
Level of Service		А	А	А	А
Degree of Saturation		0.324	0.309	0.336	0.314
Average Vehicle Delay (secs/veh)					
Beamish Street (south)	L T	2.9 0.0	2.9 0.0	2.9 0.0	2.9 0.0
Beamish Street (north)	T R	0.0 6.4	0.0 6.1	0.0 6.6	0.0 6.3
Campsie Street (west)	L R	4.7 20.0	4.4 19.1	4.7 21.5	4.4 20.1
TOTAL AVERAGE VEHICLE	DELAY	1.0	1.3	1.0	1.3
		BEA	CAMX	BEA	CAMP

In summary, those projected traffic flows as a consequence of the planning proposal will not have any unacceptable traffic implications in terms of road network capacity, nor will any road upgrades/improvements/widening be required.

Criteria for Interpreting Results of Sidra Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
Έ'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals¹ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

1

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

4. PARKING IMPLICATIONS

Existing Kerbside Parking Restrictions

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 6. Key features of those parking restrictions are:

- ¹/₂ HOUR / 1 HOUR PARKING restrictions along selected locations on Beamish Street
- BUS ZONES located at regular intervals along both sides of Beamish Street
- 1 HOUR PARKING restrictions along the Ninth Avenue site frontage
- NO PARKING / LOADING ZONE restrictions along the southern side of Ninth Avenue in the vicinity of the site
- NO STOPPING restrictions along the northern side of Ninth Avenue in the vicinity of the site
- 1 HOUR / 2 HOUR PARKING restrictions along both sides of Campsie Street, including along the entire site frontage.

Off-Street Car Parking Provisions

The off-street car parking requirements applicable to the development proposal are specified in Council's *CDCP 2012*, *Part 6.8 – Parking and Vehicle Access* document in the following terms:

	8
Studio apartments:	0.25 spaces per dwelling
1 bedroom apartments:	0.8 spaces per dwelling
2 bedroom apartments:	1 space per dwelling
3 bedroom apartments:	1 space per dwelling
Visitors:	Not required

Shop Top Housing (B2 Zones – Large Centres)



Office Premises (B2 Zones – Large Centres) 1 space per $60m^2 (120m^2 - 1,000m^2)$

Shops (B2 Zones - Large Centres)

1 space per 66.7m² (<120m²) 1 space per 33m² (120m² – 1,000m²) 1 space per 27m² (>1,000m²)

Notwithstanding, the subject site is located within 800 metres of a railway station in the Sydney metropolitan area, and therefore the residential component of the development is also subject to the parking requirements specified in the *State Environmental Planning Policy No* 65 – *Design Quality of Residential Flat Development (Amendment No 3), 2015* in the following terms:

30 Standards that cannot be used to refuse development consent or modification of development consent

- (1) If an application for the modification of a development consent or a development application for the carrying out of development to which this Policy applies satisfies the following design criteria, the consent authority must not refuse the application because of those matters:
 - a) if the car parking for the building will be equal to, or greater than, the recommended minimum amount of car parking specified in Part 3J of the Apartment Design Guide.

Reference is therefore made to the *Apartment Design Guide 2015, Section 3J – Bicycle and Car Parking* document which nominates the following car parking requirements:

Objective 3J-1

Car parking is provided based on proximity to public transport in metropolitan Sydney and centres in regional areas

For development in the following locations:

- on sites that are within 800 metres of a railway station or light rail stop in the Sydney Metropolitan Area; or
- on land zoned, and sites within 400 metres of land zoned, B3 Commercial Core, B4 Mixed Use or equivalent in a nominated regional centre

the minimum car parking requirements for residents and visitors is set out in the Guide to Traffic Generating Developments, or the car parking requirement prescribed by the relevant council, whichever is less.

The car parking needs for a development must be provided off street.

Comparison therefore needs to be drawn between the off-street car parking requirements for residential flat buildings outlined in the *Canterbury DCP 2011* and also the RMS *Guidelines* to determine the *lesser* requirement. The relevant car parking rates outlined in the RMS *Guidelines* are reproduced below:

RMS Guidelines–High Density Residential Flat Buildings in Metropolitan Regional (CBD) Centres

- 0.4 spaces per 1 bedroom unit
- 0.7 spaces per 2 bedroom unit 1.2 spaces per 3 bedroom unit
- 1 space per 7 units for visitor parking

In any event, it is expected that the above numerical car parking requirements will ultimately be satisfied as part of any future development application.

The geometric design layout of the future car parking facilities will ultimately be designed to comply with Standards Australia publication *Parking Facilities Part 1 - Off-Street Car Parking AS2890.1* and *Parking Facilities Part 6 - Off-Street Parking for People with Disabilities AS2890.6*.

Off-Street Bicycle Parking Provisions

The off-street bicycle parking requirements applicable to the development proposal are also specified in the *Canterbury DCP 2012* document in the following terms:

Residential Accommodation

Residents:	1 space per 5 dwellings or part thereof
Visitors:	1 space per 10 dwellings or part thereof

Shop, Restaurant or Cafe

Staff:	1 space per 200m ² or part thereof
Patrons:	1 space per $500m^2$ over $1,000m^2$ or part thereof

The proposed development will ultimately provide the required number of bicycle parking spaces, thereby ensuring the development's commitment to a more sustainable approach to travel.

Loading/Servicing Provisions

The proposed new development is expected to be serviced by a variety of light commercial vehicles and rigid trucks up to and including medium rigid trucks. The loading dock and manoeuvring area will ultimately be designed to accommodate the swept turning path requirements of these trucks, allowing them to enter and exit the site in a forward direction at all times, noting that the loading area will include a mechanical turntable.

The geometric design layout of the proposed loading facilities will ultimately be designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 2 - Off-Street Commercial Vehicle Facilities AS2890.2* in respect of overhead clearances, loading dock dimensions and service area requirements for MRV trucks.

Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:

- the planning proposal seeks approval to amend the planning controls of the site to permit the redevelopment of the site to comprise up to 320 apartments, approximately 3,145m² of retail floor space and 1,600m² of commercial floor space
- the capacity analysis of nearby intersections using the SIDRA capacity analysis program indicates that:
 - the projected additional traffic flows will not have any adverse effects on the operational performance of the nearby intersections, and

- no road improvements or intersection upgrades would be required as a consequence of the planning proposal
- the future car parking and loading facilities will be provided and designed in accordance with Council's requirements and the relevant Australian Standards
- the future bicycle parking facilities will be provided and designed in accordance with Council's requirements.

It is therefore reasonable to conclude that the planning proposal will not have any unacceptable implications in terms of road network capacity, access or off-street parking/loading requirements.

APPENDIX A

TRAFFIC SURVEY DATA

	R.O	.A.R.	DA	TA																			
	Relial	ble, Or	iginal	& Auth	nentic l	Result	s	PEDS	NO	RTH	WE	EST	SO	UTH	Ī	PEDS	NO	RTH	WE	EST	SOL	JTH	
	Ph.88	19684	7, Mob	.0418-	239019)		Time Per	Beam	ish St	Nintl	h Ave	Beam	ish St	тот	Peak Per	Beam	ish St	Nintl	h Ave	Beam	ish St	тот
								0630 - 0645	:	3	1	8		2	23	0630 - 0730	(6	1	00	3	3	109
								0645 - 0700		1	2	27		0	28	0645 - 0745		5	1	12	1	1	118
Clien	nt	: Varg	a Traff	ic Plan	ning			0700 - 0715		1	2	28		0	29	0700 - 0800		4	1:	30	1	l I	135
Job No/N	lame	: 6946	6 CAMI	PSIE B	Beamish	n St		0715 - 0730		1	2	27		1	29	0715 - 0815		4	1	54	1	1	159
Day/Da	ate	: Thur	sday 1	st Nov	ember	2018		0730 - 0745		2	3	30		0	32	0730 - 0830		5	1	62	()	167
								0745 - 0800	(0	4	15		0	45	0745 - 0845	1	0	1	87	()	197
								0800 - 0815		1	5	52		0	53	0800 - 0900	1	8	19	99	()	217
								0815 - 0830	1	2	3	35		0	37	0815 - 0915	2	4	1	79	()	203
								0830 - 0845	-	7	5	55		0	62	0830 - 0930	2	7	18	82	()	209
								0845 - 0900	2	8	5	57		0	65								
								0900 - 0915	-	7	3	32		0	39	PEAK HR	1	0	18	87)	197
								0915 - 0930	!	5	3	88		0	43								
								Per End	3	8	4	44		3	485								
Lights	NO	RTH	WE	ST	SOL	ЈТН		Heavies	NO	RTH	WE	EST	SO	UTH		Combined	NO	RTH	WE	EST	SOL	JTH	
	Beam	ish St	Nintl	h Ave	Beam	ish St			Beam	ish St	Nintl	h Ave	Beam	ish St			Beam	ish St	Nintl	h Ave	Beam	ish St	
Time Per	T	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	T	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	I	<u>R</u>	L	<u>R</u>	L	I	тот
0630 - 0645	63	15	46	69	99	77	369	0630 - 0645	5	0	0	1	0	5	11	0630 - 0645	68	15	46	70	99	82	380
0645 - 0700	57	7	46	65	87	57	319	0645 - 0700	5	0	0	0	1	5	11	0645 - 0700	62	7	46	65	88	62	330
0700 - 0715	58	9	47	80	65	50	309	0700 - 0715	5	0	0	1	0	6	12	0700 - 0715	63	9	47	81	65	56	321
0715 - 0730	47	15	29	62	71	57	281	0715 - 0730	5	0	0	0	2	5	12	0715 - 0730	52	15	29	62	73	62	293
0730 - 0745	55	21	61	86	77	65	365	0730 - 0745	6	0	0	0	1	4	11	0730 - 0745	61	21	61	86	78	69	376
0745 - 0800	62	25	59	79	84	71	380	0745 - 0800	6	0	0	0	2	9	17	0745 - 0800	68	25	59	79	86	80	397
0800 - 0815	66	28	55	62	71	59	341	0800 - 0815	6	0	0	0	0	5	11	0800 - 0815	72	28	55	62	71	64	352
0815 - 0830	77	25	77	86	79	89	433	0815 - 0830	3	0	0	0	0	7	10	0815 - 0830	80	25	77	86	79	96	443
0830 - 0845	97	43	83	124	89	75	511	0830 - 0845	5	0	0	0	0	6	11	0830 - 0845	102	43	83	124	89	81	522
0845 - 0900	88	27	30	90	59	71	365	0845 - 0900	4	0	0	2	1	1	8	0845 - 0900	92	27	30	92	60	72	3/3
0900 - 0915	55	37	42	58 71	55	75	332	0900 - 0915	2	0	0	1	1	2	10	0900 - 0915	62	37	42	68 72	56	60	342
Per End	811	29	4J 620	942	904	801	4359	Per End	2 59	0	0	5	8	60	132	Per End	870	281	620	947	912	861	4491
	011	201	020	012	004		1000	i di Ella	00	Ū	Ū	Ū			102	T OF EIIG	010	201	020	0-11	012		
Lights	NO	RTH	WE	EST	SO	JTH		<u>Heavies</u>	NO	RTH	WE	EST	SO	UTH		Combined	NO	RTH	WE	EST	SOL	JTH	
	Beam	ish St	Ninti	1 Ave	Beam	ish St			Beam	ish St	Ninti	1 Ave	Beam	ish St			Beam	ish St	Ninti	h Ave	Beam	ish St	
Peak Per	T	<u>R</u>		<u>R</u>		T	TOT	Peak Per	Ţ	<u>R</u>		<u>R</u>		T	TOT	Peak Per	T	<u>R</u>	L	<u>R</u>	<u>L</u>	<u>T</u>	тот
0630 - 0730	225	46	168	276	322	241	1278	0630 - 0730	20	0	0	2	3	21	46	0630 - 0730	245	46	168	278	325	262	1324
0645 - 0745	217	52	183	293	300	229	1274	0645 - 0745	21	0	0	1	4	20	46	0645 - 0745	238	52	183	294	304	249	1320
0700 - 0800	222	70	196	307	297	243	1335	0700 - 0800	22	0	0	1	5	24	52	0700 - 0800	244	70	196	308	302	267	1387
0715 - 0815	230	00	204	269	303	252	1510	0715-0815	23	0	0	0	2	25	21	0715-0815	203	00	204	209	308	2/5	1410
0730 - 0830	200	121	232	313	322	204	1665	0730 - 0830	20	0	0	0	2	23	49	0730 - 0830	201	121	232	313	314	309	1714
0800 - 0900	328	121	245	362	298	294	1650	0800 - 0900	18	0	0	2	1	19	49	0800 - 0900	346	123	245	364	200	313	1690
0815 - 0915	317	132	232	368	282	310	1641	0815 - 0915	19	0	0	2	2	16	39	0815 - 0915	336	132	232	370	284	326	1680
0830 - 0930	326	136	200	353	271	276	1562	0830 - 0930	18	0	0	3	2	14	37	0830 - 0930	344	136	200	356	273	290	1599
										-	-	-											
PEAK HR	302	121	274	351	323	294	1665	PEAK HR	20	0	0	0	2	27	49	PEAK HR	322	121	274	351	325	321	1714

	R.O.A	.R.	DAT	Ά												С	lient	: Varga Traff	ic Plan	ning	
	Reliable.	Ori	iginal &	Authe	entic F	Resul	lts									Job N	lo/Name	: 6946 CAM	PSIE B	eamish St	
DN	Ph.88196	5847	7. Mob.()418-2	39019)										Da	/Date	: Thursday 1	st Nov	ember 2018	
		-	,							1	2		3								
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										7	8		9					FOR COUNT			
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	0745 - 0	845										N									
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							Beam	ish St				T							Ream	ish St	
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									423												
						595			443										1481		
						568	0	20	⊥ I											59	
						27	121	302	•										1421		
						~ '	121	322											1 12 1	1092	
	Ninth Ave						1												60	1002	
	0	625	625-				 	+												1151	
		0	274	274		-									5	1	562	1567			
								4						Nint	h Ave	9					
								~							-				•		
		0	351	351		1						•		1193	-	185	8		Î		
•	446 44	4	2		4	∕ ∢													1773		
								321												64	
							325	294											1705		
						1	323	27												1753	
						646	2		20										68		
						617			653											1817	
						29			673												
									1											•	
					Ī																
							Beam	ish St											Beam	ish St	

	R.O	.A.R	. DA	TA																			
	Relial	ble, Or	riginal	& Aut	hentic	Resul	ts	PEDS	NO	RTH	W	ST	SO	UTH		PEDS	NO	RTH	WE	ST	SOL	JTH	
	Ph.88	19684	7, Mob	.0418-	23901	9		Time Per	Beam	ish St	Nint	h Ave	Beam	ish St	тот	Peak Per	Beam	ish St	Ninth	n Ave	Beam	ish St	тот
								1530 - 1545	1	0	6	63		0	73	1530 - 1630	2	2	24	48	4	ł	274
								1545 - 1600	;	3	6	69	;	3	75	1545 - 1645	2	1	25	53	4	1	278
Clien	t	: Varg	a Traff	fic Plar	nning			1600 - 1615	4	5	5	5		0	60	1600 - 1700	2	9	26	68	3	3	300
Job No/N	lame	: 6946	6 CAM	PSIE E	Beamis	h St		1615 - 1630		4	6	61		1	66	1615 - 1715	2	9	2	70	3	3	302
Day/Da	ate	: Thur	sday 1	st Nov	/ember	[.] 2018		1630 - 1645	9	9	6	88	(0	77	1630 - 1730	2	7	3.	13	2	2	342
								1645 - 1700	1	1	8	34	:	2	97	1645 - 1745	2	9	32	26	2	2	357
								1700 - 1715		5	5	57	(0	62	1700 - 1800	2	8	3.	19	()	347
								1715 - 1730	:	2	1	04	(0	106	1715 - 1815	3	3	34	42	()	375
								1730 - 1745	1	1	8	81		0	92	1730 - 1830	3	6	3:	33	5	5	374
								1745 - 1800	1	0	7	7		0	87								
								1800 - 1815	1	0	8	30	(0	90	PEAK HR	2	9	32	26	2	2	357
								1815 - 1830		5	9	95		5	105								
								Per End	8	5	8	94	1	1	990								
Lights	NO	RTH	WE	ST	SO	UTH		Heavies	NO	RTH	W	ST	SO	UTH		Combined	NO	RTH	WE	ST	SOL	JTH	
	Beam	ish St	Ninth	h Ave	Beam	ish St			Beam	ish St	Nint	h Ave	Beam	ish St			Beam	ish St	Ninth	n Ave	Beam	ish St	
Time Per	T	<u>R</u>	L	<u>R</u>	L	I	тот	Time Per	I	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	Ι	<u>R</u>	L	<u>R</u>	L	I	тот
1530 - 1545	95	36	43	68	82	49	373	1530 - 1545	5	1	0	0	0	4	10	1530 - 1545	100	37	43	68	82	53	383
1545 - 1600	86	45	33	68	59	57	348	1545 - 1600	8	0	0	0	0	5	13	1545 - 1600	94	45	33	68	59	62	361
1600 - 1615	68	59	37	75	63	68	370	1600 - 1615	8	0	0	0	0	4	12	1600 - 1615	76	59	37	75	63	72	382
1615 - 1630	95	50	35	55	67	71	373	1615 - 1630	8	0	0	0	0	5	13	1615 - 1630	103	50	35	55	67	76	386
1630 - 1645	93	42	39	92	88	73	427	1630 - 1645	6	0	0	0	0	4	10	1630 - 1645	99	42	39	92	88	77	437
1645 - 1700	103	32	42	75	68	68	388	1645 - 1700	2	0	0	0	0	2	4	1645 - 1700	105	32	42	75	68	70	392
1700 - 1715	69	43	44	61	74	77	368	1700 - 1715	7	0	0	0	0	2	9	1700 - 1715	76	43	44	61	74	79	377
1715 - 1730	83	59	33	68	67	79	389	1715 - 1730	4	0	0	0	2	7	13	1715 - 1730	87	59	33	68	69	86	402
1730 - 1745	69	20	47	86	99	34	429	1730 - 1745	0	1	0	1	1	6	14	1730 - 1745	70	53	47	61	100	40	443
1743 - 1800	52	29	30	50	61	90	307	1900 1915	2	0	0	0	0	0	9	1745 - 1800	57	30	30	50	61	101	300
1815 - 1830	95	37	43	61	63	86	325	1815 - 1830	2	0	0	1	2	5	10	1815 - 1830	97	37	43	62	65	91	395
Per End	1018	529	471	820	859	839	4536	Per End	62	2	0	2	5	54	125	Per End	1080	531	471	822	864	893	4661
																							1
<u>Lights</u>	NO		WE	-51	SO			Heavies	NO		Wind	-51	SO			<u>Combined</u>	NO	KIH	WE	-51	SOL		
Deels Der	Beam	ISH St	NIN	AVe	Beam	isn St	тот	Deels Des	Beam	ISH St	NIN	AVe	Beam		тот	Deels Der	Beam		NINU	AVE	Beam	5/1 5/	TOT
1520 1620	244	100	<u>L</u>	266	<u>L</u>	245	1464	1520 1620	20	<u> </u>				10	101	1520 1620	<u>1</u> 272	<u> </u>	<u>L</u>	266	<u>L</u> 271	<u> </u>	1512
1545 - 1645	344	190	140	200	271	245	1404	1545 - 1645	29	0	0	0	0	18	40	1545 - 1645	373	191	140	200	271	203	1566
1600 - 1700	359	183	153	297	286	280	1558	1600 - 1700	24	0	0	0	0	15	39	1600 - 1700	383	183	153	297	286	295	1500
1615 - 1715	360	167	160	283	297	289	1556	1615 - 1715	23	0	0	0	0	13	36	1615 - 1715	383	167	160	283	200	302	1592
1630 - 1730	348	176	158	296	297	297	1572	1630 - 1730	19	0	0	0	2	15	36	1630 - 1730	367	176	158	296	299	312	1608
1645 - 1745	365	187	166	290	308	258	1574	1645 - 1745	19	0	0	1	3	17	40	1645 - 1745	384	187	166	291	311	275	1614
1700 - 1800	330	184	160	276	308	285	1543	1700 - 1800	19	1	0	1	3	21	45	1700 - 1800	349	185	160	277	311	306	1588
1715 - 1815	314	185	155	265	295	290	1504	1715 - 1815	16	1	0	1	3	23	44	1715 - 1815	330	186	155	266	298	313	1548
1730 - 1830	326	163	165	258	291	297	1500	1730 - 1830	14	1	0	2	3	21	41	1730 - 1830	340	164	165	260	294	318	1541
PEAK HR	365	187	166	290	308	258	1574	PEAK HR	19	0	0	1	3	17	40	PEAK HR	384	187	166	291	311	275	1614

	R.O	.A.R.	DAT	ΓΑ									Clie	ent	: Varga Traff	ic Plan	ning
- 2 D AS	Reliat	ble, Ori	ginal &	& Authe	ntic Resu	ılts							Job No	/Name	: 6946 CAM	PSIE B	eamish St
DA	Ph.88	196847	, Mob.	0418-23	39019								Day/	Date	: Thursday 1	st Nove	ember 2018
								1	2		3						
								4	5		6						
														тот	AL VOLUMES		
								7	8		9			F	OR COUNT		
	PM F	PEAK													PERIOD		
	1645 ·	- 1745								N							
										A							
						Bean	nish St			V						Beam	ish St
								19									
								552									
					441			571								1364	
					424	0	19	1									64
					17	187	365									1310	
						187	384										1547
I	Ninth A	ve														54	
	1	456	457 -			⊢-l	•										1611
		0	166	166 -									2 129)1	1293		
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							275		 	_						1000	64
					▲	311	258		 							1698	1000
						308	1/		 								1838
					586	3		20	 							59	1000
					566			055	 							₽	1902
					20			6/5									
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						Bean	nish St									Beam	ish St



	R.O	.A.R.	DA	TA																			
	Relial	ble, Or	iginal	& Auth	nentic l	Result	s	PEDS	NO	RTH	W	EST	SO	JTH		PEDS	NO	RTH	W	EST	SO	JTH	
DA	Ph.88	19684	7, Mob	.0418-	239019)		Time Per	Beam	nish St	Camp	osie St	Beam	ish St	тот	Peak Per	Beam	ish St	Camp	osie St	Beam	ish St	тот
								0630 - 0645		0	3	35	(C	35	0630 - 0730	()	1	47	(<u>э</u>	147
								0645 - 0700		0	2	25	(0	25	0645 - 0745	()	1	57	(0	157
Clien	t	: Varg	a Traff	ic Plan	ning			0700 - 0715		0	4	40	(0	40	0700 - 0800		1	2	04		1	206
Job No/N	lame	: 6946	6 CAM	PSIE B	eamist	n St		0715 - 0730		0	4	47	(0	47	0715 - 0815		1	2	48		1	250
Day/Da	ate	: Thur	sday 1	st Nov	ember	2018		0730 - 0745		0	4	15	(0	45	0730 - 0830	:	3	2	65		1	269
								0745 - 0800		1	7	72		1	74	0745 - 0845	:	3	2	86		1	290
								0800 - 0815		0	8	34	(D	84	0800 - 0900	;	3	2	90		1	294
								0815 - 0830	:	2	6	64	(0	66	0815 - 0915	4	1	2	89		2	295
								0830 - 0845		0	6	56	(0	66	0830 - 0930	4	1	2	94		2	300
								0845 - 0900		1	1	76 22		1	78				2	00			204
								0900 - 0915		1 2	6	33 30		ו ר	60 71	FEAR IIK		2	2	90			294
								Per End		2 7	7	06		3	716								
																						-	
Lights	NO	RTH	WE	EST	SOL	JTH		Heavies	NO	RTH	W	EST	SO	JTH		Combined	NO	RTH	W	EST	SO	JTH	_
	Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St	-
Time Per	<u>I</u>	<u>R</u>	L	<u>R</u>	<u>L</u>	<u> </u>	TOT	Time Per	I	<u>R</u>		<u>R</u>	<u> </u>	I	TOT	Time Per	I	<u>R</u>	<u>L</u>	<u>R</u>	<u> </u>		TOT
0630 - 0645	118	14	11	9	6	106	324	0630 - 0645	5	1	0	0	1	5	12	0630 - 0645	123	15	11	9	2	1/1	336
0700 - 0715	130	9	11	7	2	125	2/4	0700 - 0715	4	1	0	0	1	5	13	0700 - 0715	135	8	10	7	3	115	205
0715 - 0730	99	12	8	12	5	118	254	0715 - 0730	5	0	1	0	0	6	12	0715 - 0730	104	12	9	12	5	124	266
0730 - 0745	135	6	14	9	11	129	304	0730 - 0745	5	1	0	0	0	5	11	0730 - 0745	140	7	14	9	11	134	315
0745 - 0800	130	10	8	5	17	148	318	0745 - 0800	6	0	0	0	1	10	17	0745 - 0800	136	10	8	5	18	158	335
0800 - 0815	115	12	16	9	13	115	280	0800 - 0815	3	3	0	0	0	5	11	0800 - 0815	118	15	16	9	13	120	291
0815 - 0830	146	15	35	9	8	136	349	0815 - 0830	3	0	0	0	0	7	10	0815 - 0830	149	15	35	9	8	143	359
0830 - 0845	210	11	19	5	11	146	402	0830 - 0845	4	1	0	0	2	6	13	0830 - 0845	214	12	19	5	13	152	415
0845 - 0900	155	23	17	11	13	115	334	0845 - 0900	6	0	0	0	2	5	13	0845 - 0900	161	23	17	11	15	120	347
0900 - 0915	115	8	14	5	11	118	271	0900 - 0915	6	1	0	0	1	3	11	0900 - 0915	121	9	14	5	12	121	282
0915 - 0930	135	20	12	6	15	113	301	0915 - 0930	3	0	0	0	2	5	10	0915 - 0930	138	20	12	6	1/	118	311
Per Ella	1001	147	162	90	115	1538	30/3	PerEnd	22	9	2	U	10	60	144	FerEna	1000	100	184	90	125	1000	3817
Lights	NO	RTH	WE	EST	SOL	JTH		<u>Heavies</u>	NO	RTH	W	EST	SO	JTH		<u>Combined</u>	NO	RTH	W	EST	SO	JTH	
	Beam	ish St	Camp	sie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St	
Peak Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	<u>T</u>	тот	Peak Per	<u>T</u>	<u>R</u>	<u> </u>	<u>R</u>	L	Ţ	тот	Peak Per	Ţ	<u>R</u>	L	<u>R</u>	L	<u> </u>	тот
0630 - 0730	460	42	47	31	16	518	1114	0630 - 0730	19	3	2	0	2	22	48	0630 - 0730	479	45	49	31	18	540	1162
0645 - 0745	4//	34	50	31	21	481	1094	0645 - 0745	19	3	2	0	1	22	4/ 52	0645 - 0745	496	37	52	31	22	503	1141
0700 - 0800	494	35	41	29	35	504	1156	0700 - 0800	10	2	1	0	2	21	53	0700 - 0800	109	37	42	29	37	536	1207
0730 - 0830	526	40	73	32	40	528	1251	0730 - 0830	17	4	0	0	1	20	<u> </u>	0730 - 0830	490 543	44	73	32	50	555	1300
0745 - 0845	601	48	78	28	49	545	1349	0745 - 0845	16	4	0	0	3	28	51	0745 - 0845	617	52	78	28	52	573	1400
0800 - 0900	626	61	87	34	45	512	1365	0800 - 0900	16	4	0	0	4	23	47	0800 - 0900	642	65	87	34	49	535	1412
0815 - 0915	626	57	85	30	43	515	1356	0815 - 0915	19	2	0	0	5	21	47	0815 - 0915	645	59	85	30	48	536	1403
0830 - 0930	615	62	62	27	50	492	1308	0830 - 0930	19	2	0	0	7	19	47	0830 - 0930	634	64	62	27	57	511	1355
PEAK HR	626	61	87	34	45	512	1365	PEAK HR	16	4	0	0	4	23	47	PEAK HR	642	65	87	34	49	535	1412

	R.O	.A.R.	DAT	Ά											Clien	t	: Varga Traf	fic Plan	ning
	Relial	ble, Ori	iginal &	Authent	ic Resu	lts									Job No/N	lame	: 6946 CAM	PSIE B	eamish St
DA	Ph.88	196847	, Mob.(0418-239	019										Day/Da	ate	: Thursday 1	st Nov	ember 2018
									1	2		3							
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																TOT	AL VOLUMES		
									7	8		9				FC	OR COUNT		
	AM F	PEAK															PERIOD		
	0800	- 0900									N								
											M								
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						Bear	nish St				V							Beam	hish St
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					622			707										1790	
					599	4	16	Ļ											64
					23	61	626	•										1720	
						65	642												1748
С	ampsi	e St																70	
	0	121	121 -		_▲ ◄	⊢–l	•												1812
		0	87	87 —										2	272		274	•	
						. A E							Camps	ie St					♦
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		0	34	34 —			↑				•	2	81	262	2	19			
<u> </u>	- 114	106	8		* ∢	-												1731	
							535												55
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					584	4		16										78	
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						Bear	nish St											Beam	ish St
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	R.O	.A.R	. DA	TA																			
	Relia	ble, Oi	riginal	& Aut	hentic	Resu	lts	PEDS	NO	RTH	W	EST	SO	JTH		PEDS	NO	RTH	WE	ST	SOL	JTH	
DN	Ph.88	19684	7, Mot	0.0418	-23901	9		Time Per	Beam	ish St	Camp	osie St	Beam	ish St	тот	Peak Per	Beam	ish St	Camp	sie St	Beam	ish St	тот
								1530 - 1545		0	1	03	(C	103	1530 - 1630		6	42	25	0)	431
								1545 - 1600		3	1	14	(0	117	1545 - 1645		6	42	29	1]	436
Clien	t	: Varg	ja Traf	fic Plai	nning			1600 - 1615		1	ę	93	(C	94	1600 - 1700	:	3	4:	37	٤	\$	448
Job No/N	lame	: 6946	6 CAM	PSIE I	Beamis	sh St		1615 - 1630		2	1	15	(C	117	1615 - 1715		7	4	61	Ċ,)	477
Day/Da	ate	: Thu	sday 1	st No	vembe	r 2018	6	1630 - 1645		0	1	07		1	108	1630 - 1730		6	4	85	ç	•	500
								1645 - 1700		0	1	22	-	7	129	1645 - 1745		6	5	15	٤	3	529
								1700 - 1715		5	1	17		1	123	1700 - 1800		6	5	56	2	2	564
								1715 - 1730		1	1	39	(0	140	1715 - 1815	:	2	5	81	1	1	584
								1730 - 1745		0	1	37	(0	137	1730 - 1830		6	5	82	3	<u>i</u>	591
								1745 - 1800		0	1	63		1	164			•		05		<u> </u>	500
								1800 - 1815		1 r	1	42	(5	143	PEAK HR		2	4	85		,	500
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Lights	NO	RTH	WE	ST	SOL	JTH		<u>Heavies</u>	NO	RTH	W	EST	SO	JTH		Combined	NO	RTH	WE	EST	SOL	JTH	
	Beam	ish St	Camp	sie St	Beam	ish St			Beam	ish St	Camp	osie St	Beam	ish St			Beam	ish St	Camp	sie St	Beam	ish St	
Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	<u>T</u>	тот	Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	L	T	тот	Time Per	<u>T</u>	<u>R</u>	L	<u>R</u>	<u>L</u>	I	тот
1530 - 1545	133	28	14	9	20	114	318	1530 - 1545	5	1	0	0	2	4	12	1530 - 1545	138	29	14	9	22	118	330
1545 - 1600	126	23	15	12	17	102	295	1545 - 1600	6	2	0	0	0	5	13	1545 - 1600	132	25	15	12	17	107	308
1600 - 1615	130	14	18	10	23	116	311	1600 - 1615	8	0	0	0	0	4	12	1600 - 1615	138	14	18	10	23	120	323
1615 - 1630	124	24	27	3	24	113	315	1615 - 1630	7	1	0	0	0	5	13	1615 - 1630	131	25	27	3	24	118	328
1630 - 1645	161	22	28	9	21	135	3/6	1630 - 1645	5	1	0	0	1	4	11	1630 - 1645	166	23	28	9	22	139	387
1700 - 1715	149	15	17	4	19	122	301	1700 - 1715	5	1	0	0	1	2	4	1700 - 1715	122	25	17	4	19	135	341
1715 - 1730	144	10	35	9	20	110	328	1715 - 1730	5	0	0	0	1	2 9	3 15	1715 - 1730	149	10	35	۱0 ۹	21	119	343
1730 - 1745	175	26	25	8	14	108	356	1730 - 1745	4	2	0	0	1	7	14	1730 - 1745	179	28	25	8	15	115	370
1745 - 1800	114	12	27	3	13	137	306	1745 - 1800	4	0	0	0	0	6	10	1745 - 1800	118	12	27	3	13	143	316
1800 - 1815	100	18	22	7	11	123	281	1800 - 1815	3	1	0	0	2	4	10	1800 - 1815	103	19	22	7	13	127	291
1815 - 1830	130	24	30	13	19	120	336	1815 - 1830	3	0	0	0	1	7	11	1815 - 1830	133	24	30	13	20	127	347
Per End	1604	240	285	97	212	1425	3863	Per End	56	10	0	0	9	59	134	Per End	1660	250	285	97	221	1484	3997
Lighte	NO	ртн	WE	ST.	501	ІТН		Hoavies	NO	PTH	W	- ет	50	ітн		Combined	NO	этн	WE	TP	501	ітн	
Lights	Beam	ish St	Camp	sie St	Beam	ish St		<u>ricavics</u>	Beam	ish St	Cam	osie St	Beam	ish St		oombined	Beam	ish St	Camp	sie St	Beam	ish St	
Peak Per	т	R	L	R	L	т	тот	Peak Per	т	R	L	R	L	Т	тот	Peak Per	т	R	L	R	L	Т	тот
1530 - 1630	513	89	74	34	84	445	1239	1530 - 1630	26	4	0	0	2	18	50	1530 - 1630	539	93	74	34	86	463	1289
1545 - 1645	541	83	88	34	85	466	1297	1545 - 1645	26	4	0	0	1	18	49	1545 - 1645	567	87	88	34	86	484	1346
1600 - 1700	564	84	100	26	87	478	1339	1600 - 1700	21	3	0	0	1	15	40	1600 - 1700	585	87	100	26	88	493	1379
1615 - 1715	552	85	99	26	75	495	1332	1615 - 1715	18	4	0	0	2	13	37	1615 - 1715	570	89	99	26	77	508	1369
1630 - 1730	572	71	107	32	71	492	1345	1630 - 1730	16	3	0	0	3	17	39	1630 - 1730	588	74	107	32	74	509	1384
1645 - 1745	586	75	104	31	64	465	1325	1645 - 1745	15	4	0	0	3	20	42	1645 - 1745	601	79	104	31	67	485	1367
1700 - 1800	551	63	104	30	58	488	1294	1700 - 1800	18	3	0	0	3	24	48	1700 - 1800	569	66	104	30	61	512	1342
1715 - 1815	533	66	109	27	58	478	1271	1715 - 1815	16	3	0	0	4	26	49	1715 - 1815	549	69	109	27	62	504	1320
1730 - 1830	519	80	104	31	57	488	1279	1730 - 1830	14	3	0	0	4	24	45	1730 - 1830	533	83	104	31	61	512	1324
PEAK HR	572	71	107	32	71	492	1345	PEAK HR	16	3	0	0	3	17	39	PEAK HR	588	74	107	32	74	509	1384

F	R.O.	A.R.	DAT	ГА											Cli	ent	: Varga Traf	fic Plar	nning
	Reliat	le, Ori	iginal &	& Authen	tic Resi	ults									Job No	/Name	: 6946 CAM	IPSIE I	Beamish St
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0		04				/4	588											50	1844
Can	npsie	51	400															59	1010
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						- 0	99 ——						Can	npsie 3	57				
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						Bean	iish St											Beam	usn St
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APPENDIX B

SIDRA MOVEMENT SUMMARIES

Site: 101 [BEA_NINX AM]

♦♦ Network: N101 [Existing Network AM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. E Qu	Back of eue	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles	Speed km/h
South	i: Bear	nish St (S)												
1	L2	325	0.6	325	0.6	0.275	7.7	LOS A	2.1	15.0	0.47	0.64	0.47	30.2
2	T1	321	8.4	321	8.4	0.620	17.9	LOS B	4.4	33.1	0.92	0.79	0.95	21.1
Appro	bach	646	4.5	646	4.5	0.620	12.7	LOS A	4.4	33.1	0.69	0.72	0.70	25.1
North	: Bean	nish St (N)												
8	T1	322	6.2	322	6.2	0.330	7.6	LOS A	2.8	20.9	0.62	0.53	0.62	26.8
9	R2	121	0.0	121	0.0	0.279	16.3	LOS B	1.3	9.4	0.84	0.74	0.84	26.4
Appro	bach	443	4.5	443	4.5	0.330	10.0	LOS A	2.8	20.9	0.68	0.58	0.68	26.6
West:	Ninth	Ave (W)												
10	L2	274	0.0	274	0.0	0.307	12.1	LOS A	2.5	17.8	0.65	0.71	0.65	29.3
12	R2	351	0.0	351	0.0	0.787	26.9	LOS B	5.7	40.0	0.99	0.98	1.24	16.1
Appro	bach	625	0.0	625	0.0	0.787	20.4	LOS B	5.7	40.0	0.84	0.86	0.98	21.8
All Ve	hicles	1714	2.9	1714	2.9	0.787	14.8	LOS B	5.7	40.0	0.74	0.74	0.80	24.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Peo	destrians						
Mov	Description	Demand	Average	Level of	Average Bacl	c of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		
P3	North Full Crossing	10	19.4	LOS B	0.0	0.0	0.88	0.88
P4	West Full Crossing	187	19.5	LOS B	0.2	0.2	0.89	0.89
All Pe	destrians	197	19.5	LOS B			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINX PM]

♦♦ Network: N101 [Existing Network PM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	lovement Performance - Vehicles													
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. E Que	Back of eue	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles	Speed km/h
South	: Bear	nish St (S)												
1	L2	299	0.7	299	0.7	0.253	7.6	LOS A	1.9	13.6	0.46	0.64	0.46	30.3
2	T1	312	4.8	312	4.8	0.516	15.4	LOS B	3.9	28.7	0.86	0.73	0.86	22.5
Appro	bach	611	2.8	611	2.8	0.516	11.6	LOS A	3.9	28.7	0.66	0.68	0.66	26.0
North	: Bean	nish St (N)												
8	T1	367	5.2	367	5.2	0.347	6.5	LOS A	3.0	22.2	0.58	0.50	0.58	28.1
9	R2	176	0.0	176	0.0	0.365	14.6	LOS B	1.9	13.0	0.82	0.75	0.82	27.4
Appro	bach	543	3.5	543	3.5	0.365	9.1	LOS A	3.0	22.2	0.66	0.58	0.66	27.7
West:	Ninth	Ave (W)												
10	L2	158	0.0	158	0.0	0.193	12.8	LOS A	1.5	10.3	0.65	0.69	0.65	28.8
12	R2	296	0.0	296	0.0	0.797	28.6	LOS C	4.9	34.5	1.00	1.00	1.30	15.5
Appro	bach	454	0.0	454	0.0	0.797	23.1	LOS B	4.9	34.5	0.88	0.89	1.07	20.0
All Ve	hicles	1608	2.2	1608	2.2	0.797	14.0	LOS A	4.9	34.5	0.72	0.71	0.78	24.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov	Deceription	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective							
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate							
		ped/h	sec		ped	m									
P3	North Full Crossing	27	19.4	LOS B	0.0	0.0	0.88	0.88							
P4	West Full Crossing	313	19.6	LOS B	0.4	0.4	0.89	0.89							
All Pe	destrians	340	19.6	LOS B			0.89	0.89							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINP AM]

♦♦ Network: N101 [Proposed Network AM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	Iovement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. B Que	ack of ue	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles I veh	Distance m		Rate	Cycles	Speed km/h
South	n: Bear	nish St (S)												
1	L2	354	0.6	354	0.6	0.299	7.7	LOS A	2.4	16.7	0.47	0.65	0.47	30.1
2	T1	321	8.4	321	8.4	0.620	17.9	LOS B	4.4	33.1	0.92	0.79	0.95	21.1
Appro	bach	675	4.3	675	4.3	0.620	12.6	LOS A	4.4	33.1	0.69	0.72	0.70	25.3
North	: Bean	nish St (N)												
8	T1	322	6.2	322	6.2	0.330	7.6	LOS A	2.8	20.9	0.62	0.53	0.62	26.8
9	R2	146	0.0	146	0.0	0.338	16.6	LOS B	1.7	11.6	0.86	0.75	0.86	26.3
Appro	bach	468	4.3	468	4.3	0.338	10.4	LOS A	2.8	20.9	0.69	0.60	0.69	26.5
West:	Ninth	Ave (W)												
10	L2	296	0.0	296	0.0	0.332	12.2	LOS A	2.8	19.5	0.66	0.71	0.66	29.2
12	R2	374	0.0	374	0.0	0.839	29.5	LOS C	6.5	45.6	1.00	1.06	1.37	15.2
Appro	bach	670	0.0	670	0.0	0.839	21.9	LOS B	6.5	45.6	0.85	0.90	1.05	21.1
All Ve	hicles	1813	2.7	1813	2.7	0.839	15.4	LOS B	6.5	45.6	0.75	0.76	0.83	23.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov	Description	Demand	Average	Level of	Average Bac	k of Queue	Prop.	Effective							
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate							
		ped/h	sec		ped	m									
P3	North Full Crossing	10	19.4	LOS B	0.0	0.0	0.88	0.88							
P4	West Full Crossing	187	19.5	LOS B	0.2	0.2	0.89	0.89							
All Pe	destrians	197	19.5	LOS B			0.89	0.89							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 101 [BEA_NINP PM]

♦♦ Network: N101 [Proposed Network PM]

Beamish St & Ninth Ave, Campsie Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Site Practical Cycle Time)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Ba Quei	ack of Je	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles C veh)istance m		Rate	Cycles	Speed km/h
South	n: Bean	nish St (S)												
1	L2	324	0.6	324	0.6	0.274	7.7	LOS A	2.1	15.0	0.47	0.64	0.47	30.2
2	T1	312	4.8	312	4.8	0.516	15.4	LOS B	3.9	28.7	0.86	0.73	0.86	22.5
Appro	bach	636	2.7	636	2.7	0.516	11.5	LOS A	3.9	28.7	0.66	0.68	0.66	26.1
North	: Bean	nish St (N)												
8	T1	367	5.2	367	5.2	0.347	6.5	LOS A	3.0	22.2	0.58	0.50	0.58	28.1
9	R2	201	0.0	201	0.0	0.419	14.9	LOS B	2.2	15.3	0.84	0.77	0.84	27.2
Appro	bach	568	3.3	568	3.3	0.419	9.5	LOS A	3.0	22.2	0.67	0.60	0.67	27.6
West:	Ninth	Ave (W)												
10	L2	169	0.0	169	0.0	0.207	12.8	LOS A	1.6	11.1	0.65	0.69	0.65	28.8
12	R2	307	0.0	307	0.0	0.827	30.0	LOS C	5.3	37.0	1.00	1.05	1.37	15.0
Appro	bach	476	0.0	476	0.0	0.827	23.9	LOS B	5.3	37.0	0.88	0.92	1.12	19.7
All Ve	hicles	1680	2.1	1680	2.1	0.827	14.3	LOS A	5.3	37.0	0.73	0.72	0.79	24.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians														
Mov	Deceription	Demand	Average	Level of	Average Back	of Queue	Prop.	Effective							
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate							
		ped/h	sec		ped	m									
P3	North Full Crossing	27	19.4	LOS B	0.0	0.0	0.88	0.88							
P4	West Full Crossing	313	19.6	LOS B	0.4	0.4	0.89	0.89							
All Pe	destrians	340	19.6	LOS B			0.89	0.89							

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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V Site: 101 [BEA_CAMX AM]

♦♦ Network: N101 [Existing Network AM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Mov	ovement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bao Queu	ck of e	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total	ΗV	Total	HV				Vehicles Di	stance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Bear	nish St (S)												
1	L2	52	5.8	52	5.8	0.177	2.9	LOS A	0.0	0.0	0.00	0.07	0.00	39.9
2	T1	573	4.9	573	4.9	0.177	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	37.9
Appro	bach	625	5.0	625	5.0	0.177	0.2	NA	0.0	0.0	0.00	0.04	0.00	38.9
North	: Bean	nish St (N)												
8	T1	617	2.6	617	2.6	0.324	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	52	7.7	52	7.7	0.066	6.4	LOS A	0.1	0.7	0.45	0.62	0.45	35.5
Appro	bach	669	3.0	669	3.0	0.324	0.5	NA	0.1	0.7	0.04	0.05	0.04	38.3
West	Camp	osie St (W)												
10	L2	78	0.0	78	0.0	0.194	4.7	LOS A	0.3	2.1	0.48	0.61	0.48	33.7
12	R2	28	0.0	28	0.0	0.194	20.0	LOS B	0.3	2.1	0.48	0.61	0.48	33.3
Appro	bach	106	0.0	106	0.0	0.194	8.7	LOS A	0.3	2.1	0.48	0.61	0.48	33.6
All Ve	hicles	1400	3.6	1400	3.6	0.324	1.0	NA	0.3	2.1	0.05	0.09	0.05	37.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMX PM]

♦♦ Network: N101 [Existing Network PM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Mov	ovement Performance - Vehicles													
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. B Que	lack of eue	Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total	HV	Total	ΗV				Vehicles	Distance		Rate	Cycles	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Bear	nish St (S))											
1	L2	74	4.1	74	4.1	0.156	2.9	LOS A	0.0	0.0	0.00	0.12	0.00	39.6
2	T1	509	5.5	509	5.5	0.156	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	37.1
Appro	oach	583	5.3	583	5.3	0.156	0.4	NA	0.0	0.0	0.00	0.06	0.00	38.7
North	n: Bean	nish St (N)												
8	T1	588	2.7	588	2.7	0.309	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	74	5.4	74	5.4	0.089	6.1	LOS A	0.1	0.9	0.43	0.61	0.43	35.7
Appro	oach	662	3.0	662	3.0	0.309	0.7	NA	0.1	0.9	0.05	0.07	0.05	38.0
West	: Camp	osie St (W))											
10	L2	107	0.0	107	0.0	0.222	4.4	LOS A	0.4	2.6	0.41	0.56	0.41	34.3
12	R2	32	0.0	32	0.0	0.222	19.1	LOS B	0.4	2.6	0.41	0.56	0.41	33.9
Appro	oach	139	0.0	139	0.0	0.222	7.8	LOS A	0.4	2.6	0.41	0.56	0.41	34.2
All Ve	ehicles	1384	3.7	1384	3.7	0.309	1.3	NA	0.4	2.6	0.06	0.11	0.06	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMP AM]

♦ Network: N101 [Proposed **Network AM]**

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Mov	ovement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Ba Que	ack of ue	Prop. Queued	Effective Stop	Aver. / No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles [Distance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Bear	nish St (S)												
1	L2	52	5.8	52	5.8	0.184	2.9	LOS A	0.0	0.0	0.00	0.07	0.00	39.9
2	T1	599	4.7	599	4.7	0.184	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	38.0
Appro	oach	651	4.8	651	4.8	0.184	0.2	NA	0.0	0.0	0.00	0.04	0.00	38.9
North	: Bean	nish St (N)												
8	T1	640	2.5	640	2.5	0.336	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	52	7.7	52	7.7	0.068	6.6	LOS A	0.1	0.7	0.47	0.63	0.47	35.4
Appro	bach	692	2.9	692	2.9	0.336	0.5	NA	0.1	0.7	0.04	0.05	0.04	38.3
West	: Camp	osie St (W)												
10	L2	78	0.0	78	0.0	0.204	4.7	LOS A	0.3	2.2	0.50	0.62	0.50	33.4
12	R2	28	0.0	28	0.0	0.204	21.5	LOS B	0.3	2.2	0.50	0.62	0.50	33.1
Appro	bach	106	0.0	106	0.0	0.204	9.2	LOS A	0.3	2.2	0.50	0.62	0.50	33.3
All Ve	ehicles	1449	3.5	1449	3.5	0.336	1.0	NA	0.3	2.2	0.05	0.08	0.05	36.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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V Site: 101 [BEA_CAMP PM]

♦♦ Network: N101 [Proposed Network PM]

Beamish St & Campsie St, Campsie Site Category: (None) Giveway / Yield (Two-Way)

Mov	lovement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. B Que	ack of ue	Prop. Queued	Effective Stop	Aver No.	Averag e
		Total	ΗV	Total	ΗV				Vehicles I	Distance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	n: Bear	nish St (S)												
1	L2	74	4.1	74	4.1	0.162	2.9	LOS A	0.0	0.0	0.00	0.11	0.00	39.6
2	T1	534	5.2	534	5.2	0.162	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	37.2
Appro	bach	608	5.1	608	5.1	0.162	0.4	NA	0.0	0.0	0.00	0.06	0.00	38.7
North	: Bean	nish St (N)												
8	T1	599	2.7	599	2.7	0.314	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	40.0
9	R2	74	5.4	74	5.4	0.091	6.3	LOS A	0.1	0.9	0.44	0.63	0.44	35.6
Appro	bach	673	3.0	673	3.0	0.314	0.7	NA	0.1	0.9	0.05	0.07	0.05	38.0
West	: Camp	osie St (W)												
10	L2	107	0.0	107	0.0	0.230	4.4	LOS A	0.4	2.6	0.43	0.57	0.43	34.1
12	R2	32	0.0	32	0.0	0.230	20.1	LOS B	0.4	2.6	0.43	0.57	0.43	33.7
Appro	bach	139	0.0	139	0.0	0.230	8.0	LOS A	0.4	2.6	0.43	0.57	0.43	34.0
All Ve	ehicles	1420	3.6	1420	3.6	0.314	1.3	NA	0.4	2.6	0.07	0.11	0.07	36.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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